

Principal Stress & Maximal Shear Stress

2-Dimensional

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\tau_{max} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\tau_{x'y'} = -\left(\frac{\sigma_x - \sigma_y}{2}\right) \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$\theta = \frac{1}{2} \tan^{-1} \left(\frac{2\tau_{xy}}{\sigma_x - \sigma_y} \right)$$

$$I_{xx} = \int y^2 dA, I_{yy} = \int x^2 dA, I_{xy} = \int xy dA, I_{xx'} = I_{xx} \cos^2 2\theta - I_{xy} \sin 2\theta + I_{yy} \sin^2 2\theta$$

3-Dimensional

Determinants Equations (Stress Invariants)

$$I_1 = \sigma_x + \sigma_y + \sigma_z$$

$$I_2 = \sigma_x \sigma_y + \sigma_y \sigma_z + \sigma_z \sigma_x - \tau_{xy}^2 - \tau_{yz}^2 - \tau_{zx}^2$$

$$I_3 = \sigma_x \sigma_y \sigma_z + 2\tau_{xy} \tau_{yz} \tau_{zx} - \sigma_x \tau_{xy}^2 - \sigma_y \tau_{yz}^2 - \sigma_z \tau_{zx}^2$$

$$I_3 = \sigma_x \sigma_y \sigma_z + 2\tau_{xy} \tau_{yz} \tau_{zx} - \sigma_x \tau_{xy}^2 - \sigma_y \tau_{yz}^2 - \sigma_z \tau_{zx}^2 = \det(\sigma_{ij})$$

$\sigma_{1,2,3}$ = roots of $\lambda^3 - I_1 \lambda^2 + I_2 \lambda - I_3 = 0$, where $\sigma_{1,2,3}$ are principal stresses

$$\sigma_{1,2,3} = \frac{I_1}{3} \pm \sqrt{\frac{I_1^2 - 3I_2}{3}}$$

See ex. for principal direction

$$\tau_{1,2,3} = \frac{I_1 \sigma_{1,2,3} - I_2}{\sigma_{1,2,3} - \sigma_{j,k}}$$

$$\tau_{1,2,3} = \frac{I_1 \sigma_{1,2,3} - I_2}{\sigma_{1,2,3} - \sigma_{j,k}}$$

Centroidal Bending

$$\sigma_{max} = \sum \frac{M y}{I} + \frac{P}{A}$$

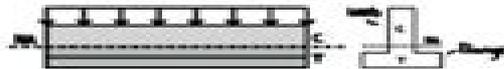
C is distance from NB to point of interest; it differs for circular shaft

I & A will be the same for both

bending stress will be the same (signified) on both

NOA is used (signified) stress distribution

tensile bending \rightarrow positive, compression bending \rightarrow negative

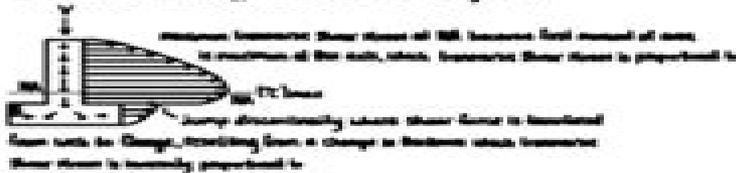


Transverse Shear

$$\tau = \frac{VQ}{It}$$

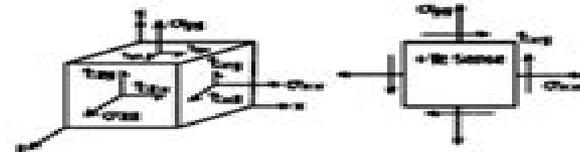
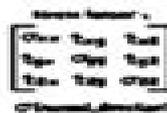
Q \rightarrow Area, for rectangles along axis but I & Q are with respect to

I & Q will be the same opposite to V (another axis (perpendicular))



Twisting

$$\tau = \frac{T\rho}{J}$$



Case Sectional Properties

$$Centroid: \bar{y} = \frac{\sum y_i A_i}{\sum A_i}$$

Second Moment of Area

$$I = \sum I_{i,centroid} + A_i d_i^2 \text{ only use if other axis is not coincident}$$

$$d_i = (y_i - \bar{y})$$

If I_{yy} , then d_i is from perspective of y -axis

If I_{xx} , " " x -axis

Area Moment of Inertia

$$J = \sum (I_{xx} + I_{yy}) \text{ if bar is used as a shaft; hollow use } I_o - I_o'$$

$$I_o = \sum (I_{xx} + I_{yy}) \text{ if bar is used as a beam}$$

First Moment of Area

moment of area above or below point of interest

$$Q = A\bar{y}$$

where A is the remaining case sectional area above or below that point

and \bar{y} is the distance between the centroid of that area to the centroid of the entire case section

2-Dimensional Plane Stress & Strain

Stress

(1) — one dimension compared to other two dimensions is very small

like a plate or a shell. Stress with large constant

(2) — shell has to be in the plane of the plate, etc., same way it does not.

Strain

(1) — one dimension compared to other two dimensions is very large

(2) — symmetric loading conditions

Stress-Strain

A: point of interest

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} \\ C_{21} & C_{22} & C_{23} \\ C_{31} & C_{32} & C_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix}$$

Stress-Strain Relationship

$$E_{11} = E(\nu^2 k_{xx} - \nu^2 k_{yy} - \nu^2 k_{zz})$$

$$E_{22} = E(\nu^2 k_{yy} - \nu^2 k_{xx} - \nu^2 k_{zz})$$

$$E_{33} = E(\nu^2 k_{zz} - \nu^2 k_{xx} - \nu^2 k_{yy})$$

$$E_{12} = E(\nu k_{xy}), E_{13} = E(\nu k_{xz}), E_{23} = E(\nu k_{yz})$$

$$C = E/(1+\nu)$$

Plane Stress Condition

$$E_{33} = E(\nu^2 k_{zz} - \nu^2 k_{xx} - \nu^2 k_{yy})$$

$$E_{23} = E(\nu k_{yz})$$

$$E_{33} = -\nu^2 E(k_{xx} - \nu^2 k_{yy})$$

$$E_{23} = E(\nu k_{yz})$$

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