

Project :- **Project**  
 Building :- **Building**  
 Element :- **Element**  
 Location :- **Location**

Designed by: **M. Abu Shady**  
 Checked by: **M. Abu Shady**  
 Date:- **8-Nov-14**

**General Input :-**

**Flat Slab Input**

Slab thk t	<b>260</b>	mm	$f_y$	<b>420</b>	N/mm <sup>2</sup>
cover	<b>25</b>	mm	$f_c'$	<b>30</b>	N/mm <sup>2</sup>
d	<b>225</b>	mm	$f_{yc}$	<b>420</b>	N/mm <sup>2</sup>

Normal weight concrete  $\lambda = 1$

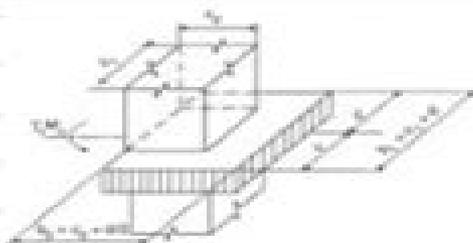
**Column Dimensions**

$c_1$ Depth	<b>600</b>	mm	
$c_2$ Vidth	<b>400</b>	mm	
Position	<b>Edge Column-width</b>	<b>Case C</b>	
$\beta$	<b>1.50</b>	ACI 11.11.2.1.a	
as	<b>30</b>	ACI 11.11.2.1.b	

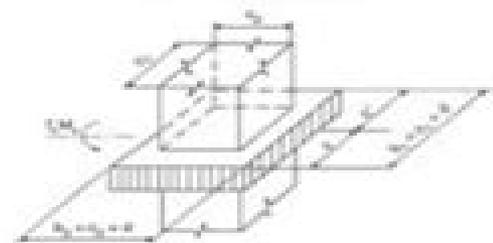
**Punching Shear Input**

$V_u$	<b>900</b>	KN
$M_{u1}$	<b>75</b>	KN.m
$M_{u2}$	<b>50</b>	KN.m

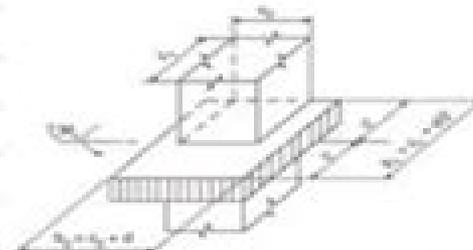
Case A: Edge Column (Bending parallel to edge)



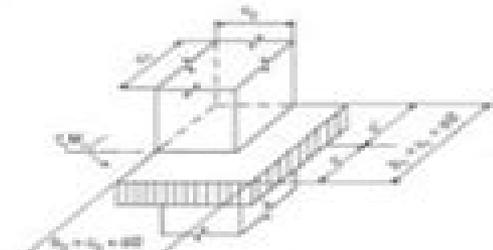
Case B: Interior Column



Case C: Edge Column (Bending perpendicular to edge)



Case D: Corner Column



**a-Calculation of RC punching shear resistance without shear reinforcement ( $\phi V_n = \phi V_c$ )**

$b_1$	<b>712.5</b>	mm	$b_2$	<b>625</b>	mm	$b_o$ substructure	<b>0</b>	mm
$b_o$	<b>2050</b>	mm			ACI 11.11.1.2			
$\phi_{shear}$	<b>0.75</b>				ACI 9.3.2.3 & ACI 9.3.4.a			
$V_c = \min. of$	$0.17 \left(1 + \frac{2}{\beta}\right) \lambda \sqrt{f_c'}$	<b>- 2.173</b>	N/mm <sup>2</sup>		ACI 11.11.2.1.a			
	$0.083 \left(\frac{a_s d}{b_o} + 2\right) \lambda \sqrt{f_c'}$	<b>- 2.406</b>	N/mm <sup>2</sup>		ACI 11.11.2.1.b			

# Checking Punching Shear Strength By The Aci Code

**AN Whitehead**



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