

in such a way that the ends of the member remain plane. Whether this can actually be accomplished is discussed at the end of Sec. 4.5.

Section 4.4 is devoted to the analysis of the normal stresses in the elastic range and presents the derivation of the *elastic flexure formulas*. It also defines the elastic section modulus and ends with the derivation of the formula for the *curvature* of an elastic beam. Section 4.5 discusses the *anticlastic curvature* of members in pure bending and also states the loading conditions required for the ends of the member to remain plane.

Section 4.6 discusses the determination of stresses in members made of several materials and defines the *transformed section* of such members. It also shows how the transformed section can be used to determine the radius of curvature of the member. The section ends with a discussion of the stresses in *reinforced-concrete beams*. Section 4.7 deals with the stress concentrations at fillets and grooves in flat bars under pure bending.

Sections 4.8 through 4.11 are optional. They discuss the plastic deformations and residual stresses in members subjected to pure bending in much the same way that these were discussed in Secs. 3.9 through 3.11 in the case of members in torsion. Section 4.8 describes the general method for the determination of the bending moment corresponding to a given maximum normal stress in a member possessing two planes of symmetry and made of a material with a nonlinear stress-strain diagram. Section 4.9 deals with members made of an *elastoplastic material* and derives formulas relating the thickness of the elastic core and the radius of curvature with the applied bending moment in the case of members with a rectangular cross section. It also defines the *shape factor* and the *plastic section modulus* of members with a nonrectangular section. Section 4.10 deals with

the determination of the plastic moment of members made of an elastoplastic material and possessing a single plane of symmetry, while Sec. 4.11 discusses *residual stresses*.

Section 4.12 shows how the stresses due to a *two-dimensional eccentric axial loading* can be obtained by replacing the given eccentric load by a centric load and a couple, and superposing the corresponding stresses. Attention is called to the fact that the neutral axis *does not pass* through the centroid of the section.

Section 4.13 deals with the *unsymmetric bending* of elastic members. It is first shown that the neutral axis of a cross section will coincide with the axis of the bending couple if, and only if, the axis of the couple is directed along one of the principal centroidal axes of the cross section. It is then shown that stresses due to unsymmetric bending can always be determined by resolving the given bending couple into two component couples directed along the principal axes of the section and superposing the corresponding stresses.

This method of analysis is extended in Sec. 4.14 to the determination of the stresses due to an *eccentric axial loading in three-dimensional space*. The eccentric load is replaced by an equivalent system consisting of a centric load and two bending couples, and the corresponding stresses are superposed.

Section 4.15 is optional; it deals with the bending of *curved members*.

## Chapter 5 Analysis and Design of Beams for Bending

In Sec. 5.1 beams are defined as slender prismatic members subjected to transverse loads and are classified according to the way in which they are supported. It is shown that the internal forces in any given cross section are equivalent to a shear force  $V$  and a bending couple  $M$ . The

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